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(Vulnerability)

EMS-98

7

(Qualitative Method)

Vulnerability, and Expected Seismic Performance of Buildings in West Bank, Palestine

Absract there are different factors affecting the over all vulnerability of a structure in addition to its construction type. These factors are generally applicable to all types of structures. To emphasize the necessary data required for assigning the vulnerability classes for Palestinian buildings, seven , represents the almost the main regions in West Bank, were investigated by collecting information based on the site conditions, regularity and configuration structural and architectural elements of buildings, adjacency, edge material conditions ,etc.

For each city, two representative zones or more were selected for the investigation. The collected data and analysis were determined according to European Macroseismic scale 1998 (EMS) and calibrated by using Japanese qualitative method. The results showed that one third of the investigated buildings belong to seismic vulnerability of class A (Many buildings of class A will suffer heavy damage), whereas about 40 percent of the buildings indicate class B (Many buildings of class B will suffer moderate damage).

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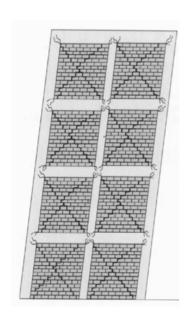
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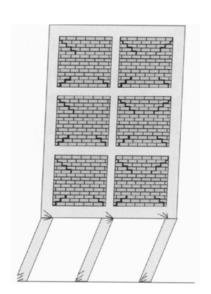
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.(2)









(2)

(Seismic Joints)

.(3





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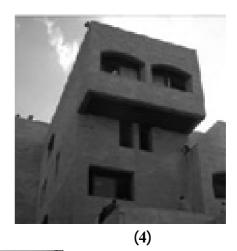
(3)

(Cantliever Systems)) (Spans)

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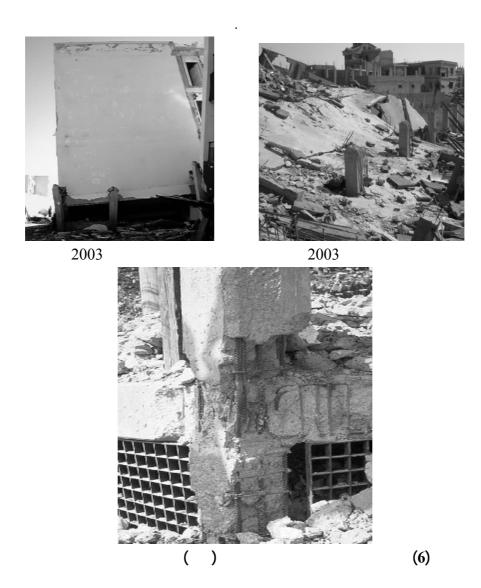
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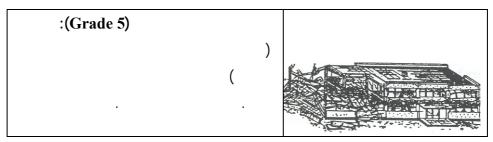
(Confinement)
(/ , (6)



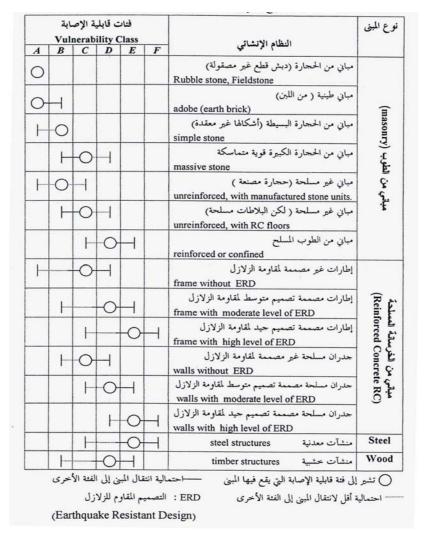
.3 (Vulnerability) MM (Medvedev Sponbeuer Karink) MSK (Modified Mercalli) (Magnitude) M MM.M1.3 [7] (European Macroseismic Scale) (Masonry Buildings) .(8 7) EMS - 98 1.4 : [7] (Building Types) [8] EMS-98 (Vulnerability Classes) (9) A 6 EMS-98 (7)

تصنيف االإنهيار في مباني الطوب (المباني غير المسلحة) Classification of damage to masonry building										
(Damage of grade :										
		1))								
	()								
(Damage of grade 2) :)								
(Damage of grade 3) :										
		(E E E E E							
(Damage of grade 4) :	. (AX BY							
(Damage of grade 5) : ().									

تصنيف الانهيار في المباني المسلحة										
Classification of damage to buildings of reinforced concrete										
:(Grade1)) (
:(Grade 2)										
. ()										
:(Grade 3)										
. (
:(Grade 4)										
) (



EMS-98 :(8)



EMS98 (9)

:					3.3
(Seismic Intensity)					
EMS- 98					
:				EN	MS- 98
		:	:		3.4
		•	•		
·					
		••		п	
(5)				•	
(Damage of grade4) 4			A		
	.5				
(Damage of grade3) 3			В		
	.4				
(Damage of grade 2)2			C		
	.3				
.(Damage of grade 2) 2			D		
	:		:		3.5
					(
					(
					•
.(Damage of grade 5) 5			A		(
4			В		(
4			D	2	-
2			C	.5	,
3			С		
				.∠	ł

: 3.2

(Vulnerability Classes)

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[14] [13] [12] [11] [10] [9] [8] [7] [6] (Regular Buildings)

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د. جلال الدبيك
                                                                   .B
            ( Reinforced concrete frame buildings)
                    C
                                          (Braced buildings)
             ) D
       .(
                         Shear walls
                                                  .(9
       (rigidity)
                                            (
                                                         )
                                 C
         A
                        В
       (2
                  )
                                                          (Stiffness)
                                 /
                                                               :
(Variation in Stiffness)
                                          В
                 )
                                                                    (2
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(H) : (L) (M)

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. EMS-98

(2)

D C B A

E F

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A	الإصاباً B	، قابلية C	فنة D	معامل الأهمية (I)	تشكيل المدخل الرنيسي	وجود أنظمة الطيران	وجود أعمدة قصيرة	وجود طابق رخو	فواصل زلزالية	عدم تماثل عمودي	عدم تماثل أفقي	نسبة التحافة	حالة المبنى (المواد)	نوع التربة	إتحدار الموقع	التصميم الزئزالي	نوع البناء	عوامل رمز المبنى	الرقم
				1.2	unsafe	H-W _H	M	Н	-	M	L	8	G	Sc	M	Without	R.C-Mas	Nablus\Z213	7.1
				1	Safe	L-W _L	L	-	-	L	-	<3	E	SB	-	Without	R.C-Mas	Nablus\Z214	7.7
				1	unsafe	L-W _L	M	-	-	M	-	<3	G	S _B	-	Without	Masonry	Nablus\Z215	7.7
				1	Safe	M-W _M	L	L	d=2cm	Н	M	<3	В	S _B	M	Without	Masonry	Nablus\Z216	7.5
				1	unsafe	j-	L	-	d=1cm	M	-	<3	G	Sc	L	Without	Old Masonry	Nablus\Z217	7.0
_				1	Safe	-	L	-	-	-	L	<3	V.G	SA	L	Without	R.C-Mas	Nablus\Z218	٣.٦
_				1	Safe	L-W _L	L	L		L	M	<3	V.G	SA	L	Without	R.C-Mas	Nablus\Z219	T.V
٠				1	unsafe	-	L	L	-	L	M	5	G	SA	L	Without	R.C-Mas	Nablus\Z220	٣٠٨
		_		1	Safe	H-W _H	L	L	d=2cm	L	Н	<3	G	SB	M	Without	R.C-Mas	Nablus\Z221	٣.٩
	<u></u>			1	Safe	M-W _L	-	-	-	L	L	<3	V.G	SB	L	Without	Masonry	Nablus\Z222	71.
_				1	Safe	M-W _L	L	-	-	-	-	<3	G	SB	L	Without	Masonry	Nablus\Z223	711
_				1	Safe	-	L	-	-	L	L	7	G	SB	M	Without	R.C-Mas	Nablus\Z224	717
	_			1	Safe	-		-	-	-	-	<3	V.G	S _B	L	Without	R.C-Mas	Nablus\Z225	717
				1	Safe	M-W _H	M	M	d=1cm	M	M	<3	V.G	S _B	M	Without	Masonry	Nablus\Z226	715
				1	Safe	-			- '	-	-	<3	V.G	SB	-	Without	Old Masonry	Nablus\Z227	710

L: Low M: Moderate E: Excellent V.G: Very Good I= 1, Normal, Residential Buildings. I=1.2, Hazardous Buildings, Schools, Hospitals. SA: Hard Rock. SB: Rock.

H: High WL: Low weight G: Good B: Bad

I= 1.5, Essential Buildings, Power- Generating

Sc: Very dense soil and soft rock. R.C.Mas: Reinforced concrete beams and columns with exterior

W_M: Moderate weight

V.B: Very Bad

stations, All structures with occupancy grater than 500 Persons.

WH: Heavy weight

decorative masonry walls.

ERD: Earthquake Resistance Design

(-): Not applied or no effect for the mentioned

(2)

				(-)
D	C	В	A	
3%	17%	41%	39%	820
7%	22%	39%	23%	120
0%	26%	31%	43%	120
0%	12%	43%	45%	100
0%	21%	45%	34%	100
3%	19%	37%	41%	80
0%	19%	39%	42%	100

8 7

9 - 8

.(3)

(3)

 							(3)			
7:				8:			9:			
5	4	3	5	4	3	5	4	3		
			5.9%	21%	18%	15.6%	23%	15%		
-	_	1	4.8%	19%	19%	-	_	_		
-	-	-	6.5%	22%	17%	-	-	-		
-	-	-	6.75%	24%	19%	-	-	-		
-	5.1%	20%	5.1%	20%	21%	-	-	-		
_	6.15%	22%	6.15%	22%	18%	-	_	_		
_	-	-	6.3%	23%	19%	-	_	_		

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د. جلال الدبيك
                                                                     12
9
                                                  :
                 E_{\text{T}}
                                            I_{s}
                                                                                  )
                                                              20
                                         I_s > E_T:
                                                            (Capacity > demand)
   I_s = E_o G So T

E_o = \Phi C F
                                                                                  (1)
                                                                                  (2)
  \begin{split} E_o: & \text{the basic seismic index} \\ G: & \text{the geological index} \end{split}
   SD: the structural design index
  T: the time index
   \boldsymbol{\Phi}\, : the story index
   C: the strength index
   F: the ductility index
                                                                                                    G
       .1
                   G
                                                                              I_{s} \\
                                                                                    (10
                                                                                                         )
```

 E_{o}

.[15]

This seismic protection index E_T is a one level index and can be Estimated the following equation:

$$E_S = (C_R F) \cdot (a_g \dots 1.4$$

$$C_R$$
 .F = 3.15 $\sqrt{\frac{T_g}{2T}} \le 3.15$ for flexural yielding type

buildings

$$C_R.F = 2.90 \sqrt{\frac{T_g}{2T}} \le 3.15$$
 for flexural yielding type

buildings Where

 E_s : the basic seismic protection index G_G : correction factor for topography

C_I: importance factor

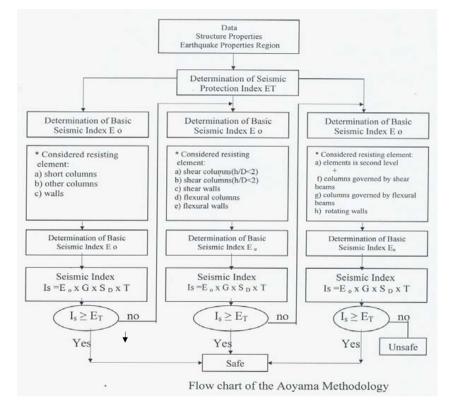
C_R: strength ratio (strength divided by the mass and ground peak acceleration)

F : the ductility index

 a_g/g : the ground peak acceleration divided by gravity acceleration

 $\begin{array}{ll} T_g & : \ \ perdominant\ period\ of\ the\ ground \\ T & : \ natural\ period\ of\ the\ building \end{array}$

(Amr, 1998)



(10)

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د. جلال الدبيك
                                                         E_{T}
E_T = E_s G_G C_1
                                                        (3)
E_s = (C_RF). (a_g/g)
                                                        (4)
                          E_{\text{T}}
GG
                                                  E_{s}
                                                       [15]
                                                        I
                  CR
                                       F
    (2
                       )
          (1)
                                           EMS-98
                                                                 20
[2]
```

0.20 0.15 a_g/g (0.25 - 0.2) (4

(4) C
)B9 B7 B6 B3 B2 B1
B8 B5 (
(10)
B3 B8 B5

С

(10) (4)

.

(T)

(4)

							(1)
I_s	Q	С	F	T_{T}	S_D	G	Build
0.21	1	1.2	0.45	0.7	0.7	0.0	D.1
0.21	l	1.2	0.45	0.7	0.7	0.8	B1
0.21	1	1.25	0.45	0.7	0.8	0.67	B2
0.28	1	1	0.45	0.7	0.9	1	В3
0.27	1.25	1.5	0.5	0.6	0.6	0.8	B4
0.34	1.25	1.25	0.55	0.7	0.7	0.8	B5
0.28	1.25	1.5	0.5	0.5	0.6	1	B6
0.40	1	1.25	0.55	0.8	0.8	0.9	B7
0.42	1	1.2	0.55	0.9	0.7	1	B8
0.37	1	1.35	0.55	0.8	0.7	0.9	В9

V _{ul}	Eval	E_{T}	A_{g}	T	T_{T}	C_{I}	G_{G}	Build
			g					
Α	No	0.38	0.12	0.2	0.5	1	1	B1
Α	No	0.38	0.12	0.3	0.6	1	1	B2
С	Y	0.29	0.12	0.25	0.3	1	1	В3
Α	No	0.51	0.24	0.4	0.3	1	1.2	B4
В	No*	0.40	0.24	0.9	0.3	1	1.3	В5
Α	No	0.46	0.24	0.5	0.3	1	1.2	В6
В	No	0.53	0.24	0.5	0.4	1	1.1	В7
В	No*	0.436	0.24	0.45	0.4	1	1	В8
A	No	0.54	0.24	0.35	0.35	1	1.1	В9

(Amr, 1998 and Mario 1994)

 $:E_{va}$

 $:\!V_{ul}$

:No*

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EMS-98 .

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